

GENERAL NOTES

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- G1. This drawing shall be read in conjunction with the architectural plans and specifications.
- G2. The builders is to check and be responsible for the correctness of all dimension and any discrepancy is to be reported. Do not scale drawings.
- G3. Stability of the building during construction and all excavations in the vicinity of neighbouring buildings is the sole responsibility of the builder.
- G4. All workmanship and materials are to be in accordance with current AS codes BCA and local council requirement.
- G5. Design Live Loads are follows.
 - Roof Live Load 0.25 kPa
 - Floor Load 1.50 kPa Deck / Balcony Load 2.00 kPa
 - Roof Snow Load 5 kPa
- G6. Plans which have been stamped and approved by building surveyor or relevant authority shall be used for construction.
- G7. Omega project services is not responsible for the professional indemnity insurance for design of pool.

STEEL :

- S1. All steel work shall be in accordance with AS4100.
- S2. Welding shall be minimum 6mm Continuous Fillet Weld (Category SP) E41XX/W40X or Complete Penetration Butt Weld (Category SP) except at toes of rolled steel sections where they shall be maximum size permitted by the welding code. All butt welding to develop the full strength of minimum member connected. U.N.O.
- S3. Bolts shall be mild steel in 2.0mm clearance holes. Where High Strength Structural Bolts (i.e.8.8/S) are specified they shall be in accordance with AS/NZS1252 and tightened by an approved method.
- S4. Steelwork shall be given one shop coat of primer except that none shall be applied at contact surface where 8.8/S bolts are used.
- S5. The fabricator shall submit shop detail drawings to the engineer for approval of connection before commencing fabrication. These shall be in accordance with the publications from Australian Steel Institute (ASI).
- S6. The fabricator shall provides all cleats and holes for the connection of purlins, girts brick ties, etc. Braces and ties shall have true intersection.
- S7. Ties and braces shall be connected to 10mm gusset plates with 2-12mm Ø bolts each end.
- S8. Steel work below floor level shall be encased 75mm minimum in concrete.
- S9. Column shall be bedded on 1:1 cement grout after plumbing and leveling on steel packers.
- S10. Members bent during fabrication, transport or handling will not be accepted.
- S11. Provide sag rods and / or struts to manufacturer's rec, to purlins and girts.
- S12. Steelwork where concrete is encased shall be wrapped with SL62 mesh and have 50mm cover.
- S13. Steel work to be protected at exposed areas as per BCA, clause 3.4.4.2. Refer to BCA for corrosion specifications. U.N.O.
- S14. Steel lintels to have a minimum end bearing of 230mm. U.N.O.
- S15. The grade of Open Section shall be Grade 300 and Hollow Section Grade 450 as a minimum.

CONCRETE :

- C1. Concrete quality as per AS3600 shall be as follows
28 days type testing
F'c = 30MPa maximum slump 75mm and maximum aggregate size of 20mm. U.N.O.
- C2. Concrete cover to all reinforcement (finishes not include)
- | Element | Sheltered | Exposed | No-Form |
|---------------|-----------|---------|---------|
| Slabs & Walls | 20mm | 30mm | 65mm |
| Beams | 25mm | 40mm | 65mm |
| Columns | 40mm | 50mm | 75mm |
| Footing | -- | 65mm | 75mm |
- C3. Bar and Mesh designations
- Ø - Structural grade round bar to AS1302
 - N - Structural grade deformed bar to AS1302
 - H - Hard grade deformed bar to AS1302
 - CW - Cold Worked deformed bar to AS1302
 - F - Fabric to AS1303 and AS1304

Element	Sheltered	Exposed	No-Form
Slabs & Walls	20mm	30mm	65mm
Beams	25mm	40mm	65mm
Columns	40mm	50mm	75mm
Footing	--	65mm	75mm

- C3. Bar and Mesh designations
 - Ø - Structural grade round bar to AS1302
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- C4. All concrete shall be mechanically vibrated and the vibrator shall not be used to vibrate the forms nor shall it be used to spread concrete.
- C5. Depth of beam is given first and includes slab thickness not including any finishes that may be applied.
- C6. Splices in reinforcement shall be sufficient to develop the full strength of the reinforcement without displacement from structural location. Laps to fabric shall be two transverse wires plus 100mm.
- C7. Reinforcement shall be accurately and firmly fixed in position shown to give support and cover specified during all operations of pouring, etc.
- C8. Conduits, pipes, etc., must not be placed in concrete cover and no holes other than those shown on the drawings shall be permitted.
- C9. Form work shall remain in position for a min. 14 days and where slabs and beams are to support brickwork over, form work shall remain up to min. 28 days and then props must be removed prior to commencement of this brick work.
- C10. Concrete must be cured for 7 days using water ponding wet sand or otherwise approved methods of adequate curing unless noted otherwise.
- C11. Wheelbarrow or pump pipes must be supported directly from the form work.
- C12. Additives must not be added to concrete without the engineer's approval.
- C13. Field welding of reinforcement is only permitted where shown on the drawings or otherwise approved.
- C14. Load-bearing brickwork shall be separated from concrete by the using of Malthoid or similar.

PREPARATION OF SUB-BASE FOR SLABS ON GROUND

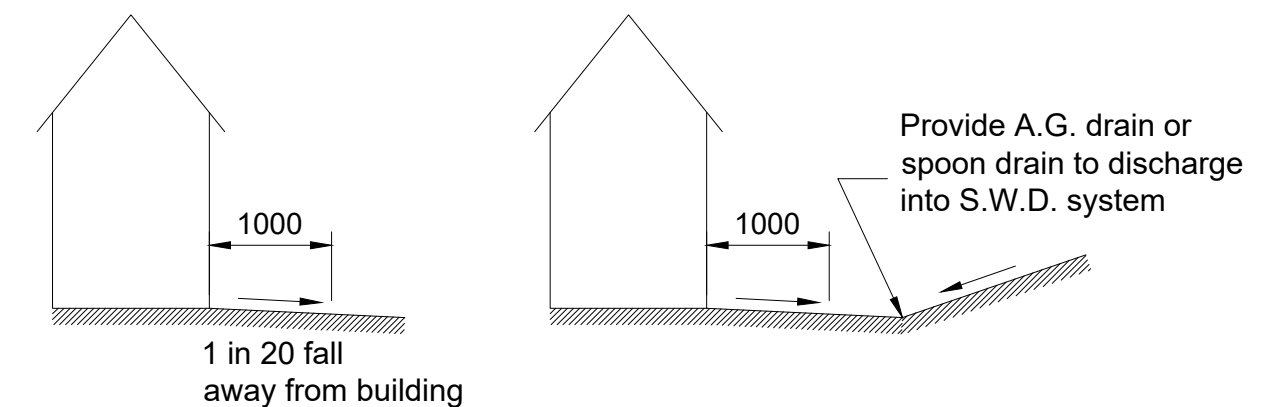
- P1. All preparation of sub-base of slabs on ground to be in accordance with AS2870.
- P2. Clear area under slab of all top soil containing humus and vegetable matter 100mm minimum.
- P3. Provide fill under slab where required to produce finished levels as shown on plans. All fill shall be imported and conform R.C.A. standard specification for Class 3 crushed rock (20mm nominated size).
Fill to be compacted in 150mm maximum layers to 95% of the modifies max dry density (M.M.D.D.) when tested in accordance with AS1289.
- P4. The upper layer of the cut surface shall be within 85% to 115% of optimum moisture content and to be properly compacted to 95% M.M.D.D..
- P5. A 50mm minimum base course of packing sand shall be spread over the sub-base and to the thoroughly rolled and compacted to a smooth level surface.
The sand shall be moistened prior to the placement of a 0.2mm polythene membrane in 3600mm minimum wide sheets lapped 150mm and jointed with 75mm wide pressure sensitive tape. The tape shall be laid under all slabs and walls in contact with the ground.
- P6. The total fill beneath the slab panels shall be in accordance with Geotechnical Report or less than 300MM, i.e. the sum of existing fill plus any new filling placed together as per Geotechnical Report or must not exceed 300mm as a maximum. Fill to be located as per clause 6.4.2 AS2870.
- P7. All the foundation is designed at 100 kpa allowable bearing capacity. Contractor to verify this prior to execution.
- P8. Ground water is not encounter at site. However, the foundation design shall be revised accordingly in the presence of ground water.

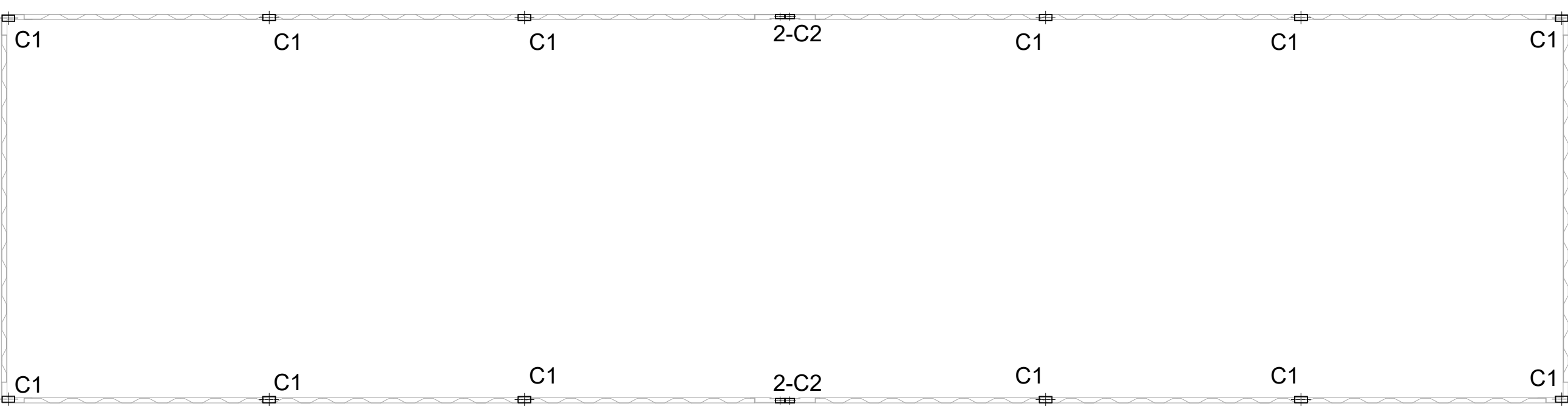
INSPECTIONS :

No responsibilities shall be taken unless the work is inspected and approved during construction. All inspections required shall be confirmed 72 hours in advance of time required.

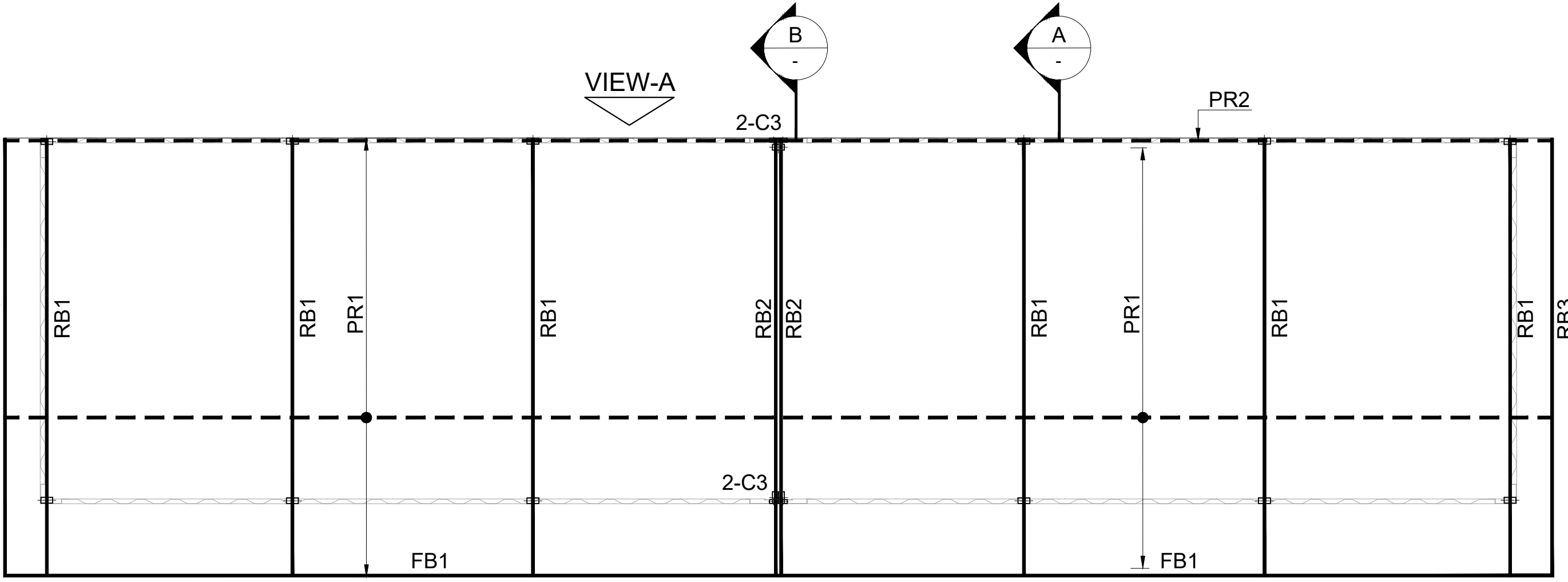
SITE DRAINAGE :

Site should be drained so that water cannot pond against or near the building. The ground immediately adjacent to the building should be graded to fall 50mm over the first meter. Where this is impracticable (i.e. on several sloping sites) use A.G. drains adjacent to footing where ground falls towards the building. (Refer to detail below). Good drainage and paving around house is recommended to ensure long term performance of building. All dimensions shown below are minimum or as per Geotechnical Report, whichever is GREATER.

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COLUMN LAYOUT PLAN
SCALE - 1:50



ROOF FRAMING PLAN
SCALE - 1:50

COLUMN SCHEDULE

COLUMN MARK	COLUMN SIZE	REMARKS
C 1	100 x 50 x 3 SHS STEEL POST	POST WELDED TO BASE
C 2	EXISTING RHS POST	
C 3	100 x 50 x 3 RHS STEEL POST	POST WELDED TO TOP OF THE EXISTING RHS TUBES/FRAME

BEAM SCHEDULE

BEAM MARK	BEAM SIZE	REMARKS
RB1	150x100x4 RHS	
RB2	150x50x3 RHS	
RB3	150x50x3 RHS	
FB1	150x50x3 RHS	FACIA BEAM

PURLIN SCHEDULE

PURLIN MARK	PURLIN SIZE
PR1	C 100-19 @ 600 CTS
PR2	C 150-1.2

ROOF MEMBERS HAVE BEEN DESIGNED FOR
A CHARACTERISTIC SNOW LOAD OF 5.0 kPa

WIND CLASSIFICATION N3

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